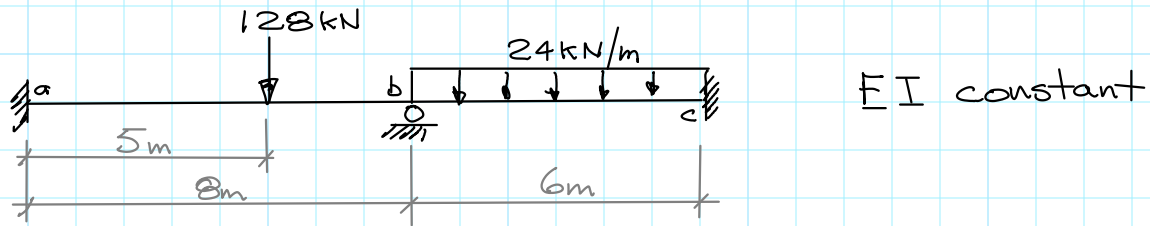


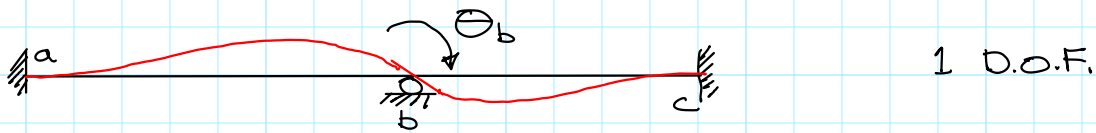
Beam Example 1 (In Class Nov 28, 30)

Method of Slope Deflection

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Step 1) Identify & label the independent joint displacements (DOF) - these must be solved for



Step 2) Determine the fixed-end moments; these are the moments that would be in each member end if they were all fixed against displacement (rotation, in this case).

$$M_{ab}^f = -\frac{Pab^2}{L^2} = -\frac{128 \text{ kN} \times 5 \text{ m} \times (3 \text{ m})^2}{(8 \text{ m})^2}$$

$$= -90 \text{ kN}\cdot\text{m}$$

$$M_{ba}^f = \frac{Pa^2b}{L^2} = \frac{128 \text{ kN} \times (5 \text{ m})^2 \times 3 \text{ m}}{(8 \text{ m})^2}$$

$$= +150 \text{ kN}\cdot\text{m}$$

$$M_{bc}^f = -\frac{wL^2}{12} = \frac{24 \text{ kN/m} \times (6 \text{ m})^2}{12}$$

$$= -72 \text{ kN}\cdot\text{m}$$

$$M_{cb}^f = \frac{wL^2}{12} = \frac{24 \text{ kN/m} \times (6 \text{ m})^2}{12}$$

$$= +72 \text{ kN}\cdot\text{m}$$

Step 3)

Write slope-deflection equations.

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These express moments at the ends of each segment in terms of the unknown joint displacements identified in step 1).

$$M_{ab} = \frac{EI}{L} \left(4\theta_a + 2\theta_b - 6 \frac{\Delta_{ab}}{L} \right) + M_{ab}^f$$
$$= \frac{EI}{8m} (2\theta_b) - 90 \text{ kN-m}$$

$$= \frac{EI}{4m} \theta_b - 90 \text{ kN-m}$$

$$M_{ba} = \frac{EI}{L} \left(2\theta_a + 4\theta_b - 6 \frac{\Delta_{ab}}{L} \right) + M_{ba}^f$$

$$= \frac{EI}{8m} (4\theta_b) + 150 \text{ kN-m}$$

$$= \frac{EI}{2m} \theta_b + 150 \text{ kN-m}$$

similarly

$$M_{bc} = \frac{EI}{6m} (4\theta_b) - 72 \text{ kN-m}$$

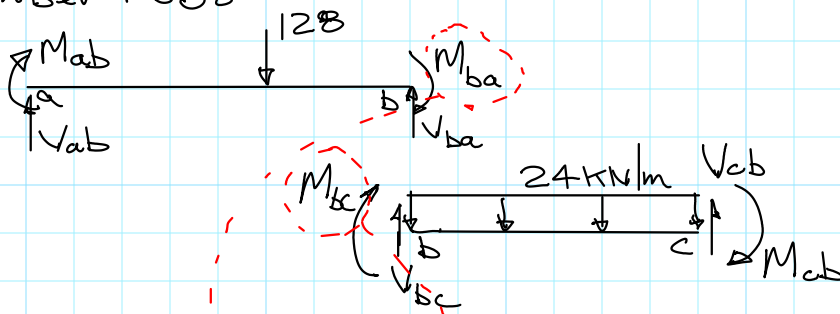
$$= \frac{2EI}{3m} \theta_b - 72 \text{ kN-m}$$

$$M_{cb} = \frac{EI}{6m} (2\theta_b) + 72 \text{ kN-m}$$

$$= \frac{EI}{3m} \theta_b + 72 \text{ kN-m}$$

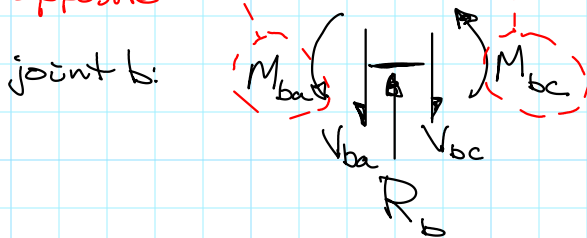
Step 4) Write equilibrium equations involving the end moments (and therefore also the unknown displacements)

Member FBDs'



joint FBD's (forces are equal & opposite to member end forces)

equal & opposite



$$\sum M_b = 0 \quad (+\curvearrowright)$$

$$M_{ba} + M_{bc} = 0$$

$$\frac{EI}{2m} \theta_b + 150 \text{ kN-m} + \frac{2EI}{3m} \theta_b - 72 \text{ kN-m} = 0$$

$$\frac{EI}{m} \left(\frac{1}{2} + \frac{2}{3} \right) \theta_b + 78 \text{ kN-m} = 0$$

Step 5) Solve for the joint displacements

$$\theta_b = \frac{-78 \text{ kN-m}^2}{EI} \times \left(\frac{1}{\frac{1}{2} + \frac{2}{3}} \right)$$

$$\theta_b = \frac{-468 \text{ kN-m}^2}{7EI} \quad (\because \curvearrowleft)$$

Step 6) Back substitute displacements into slope deflection equations to obtain member end moments.

$$M_{ab} = \frac{EI}{4m} \times \frac{-468 \text{ kN-m}^2}{7} - 90 \text{ kN-m} = -106.7 \text{ kN-m} (\therefore \downarrow)$$

$$M_{ba} = \frac{EI}{2m} \times \frac{-468 \text{ kN-m}^2}{7EI} + 150 \text{ kN-m} = 116.6 \text{ kN-m} (\therefore \downarrow)$$

$$M_{bc} = \frac{2EI}{3m} \times \frac{-468 \text{ kN-m}^2}{7EI} - 72 \text{ kN-m} = -116.6 \text{ kN-m} (\therefore \downarrow)$$

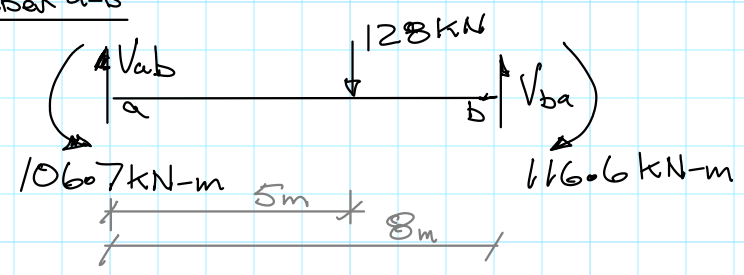
$$M_{cb} = \frac{EI}{3m} \times \frac{-468 \text{ kN-m}^2}{7EI} + 72 \text{ kN-m} = +49.7 \text{ kN-m} (\therefore \downarrow)$$

we observe that $M_{ba} + M_{bc} = 0$, as it should. this is a check that we solved the eqns properly. It does not ensure that the eqns were correct - that comes later.

Step 7) Determine member end shears & reactions using statics.

Draw FBD of each member

member a-b



$$\sum M_a = 0 \quad (+\curvearrowright)$$
$$-128 \times 5 - 116.6 + 106.7 + V_{ba}(8) = 0$$

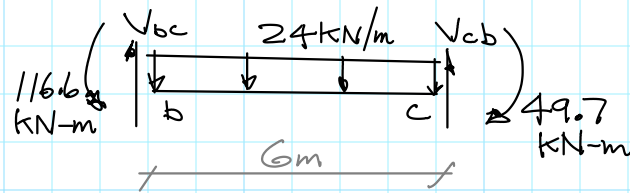
$$V_{ba} = 81.2 \quad (\therefore \uparrow)$$

$$\sum F_y = 0 \quad +\uparrow$$
$$V_{ab} - 128 + 81.2 = 0$$

$$V_{ab} = 46.8 \quad (\therefore \uparrow)$$

member bc

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$$\sum M_b = 0 \quad (+\curvearrowright)$$

$$-24 \frac{\text{kN}}{\text{m}} \times 6\text{m} \times 3\text{m} + V_{cb}(6\text{m}) - 49.7 \text{ kN-m} + 116.6 \text{ kN-m} = 0$$

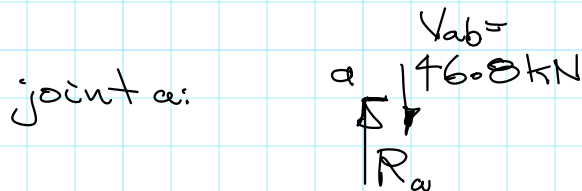
$$V_{cb} = 60.8 \text{ kN} \quad (= \uparrow)$$

$$\sum F_y = 0 \quad +\uparrow$$

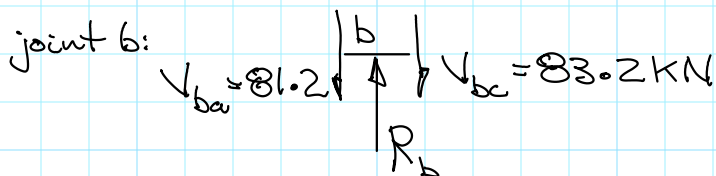
$$V_{bc} - 24 \frac{\text{kN}}{\text{m}} \times 6\text{m} + 60.8 \text{ kN} = 0$$

$$V_{bc} = 83.2 \text{ kN} \quad (= \uparrow)$$

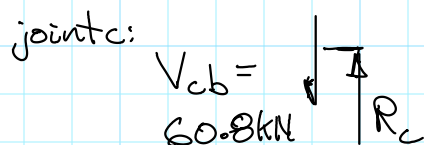
Now from joint FBD's (observing equal & opposite nature of shears) obtain the reactions.



$$R_a = 46.8 \text{ kN}$$

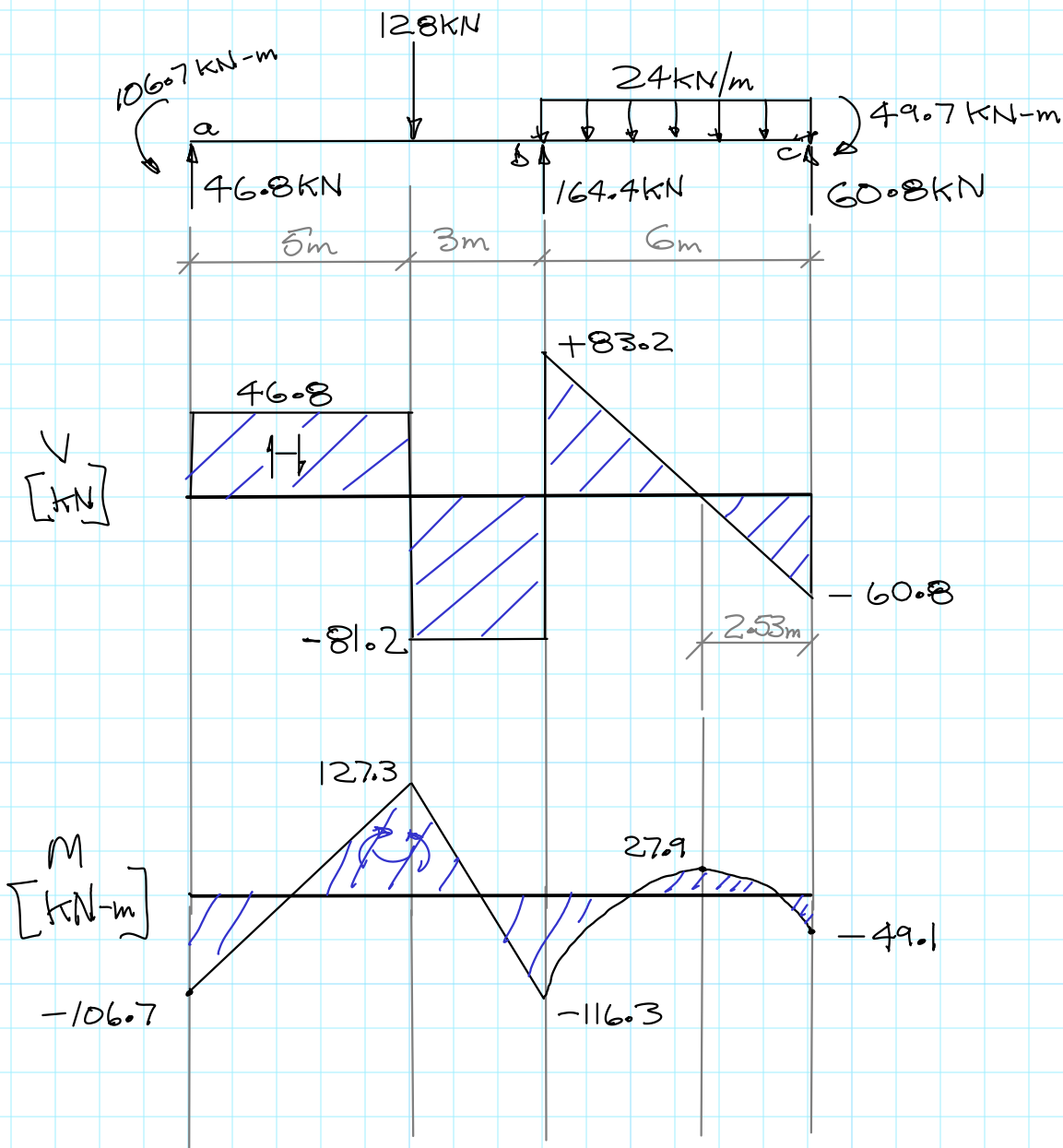


$$R_b = 81.2 \text{ kN} + 83.2 \text{ kN} = 164.4 \text{ kN}$$



$$R_c = 60.8 \text{ kN}$$

Step 8) Complete FBD, V & M diagrams



The values on the M diagram were determined by integrating the V diag. Compare with those compute in Step 6)

$$M_{cb} = 49.7 \text{ there} \\ 49.1 \text{ here} \quad \text{Ok.}$$