

Principle of Virtual Displacements (Bernoulli 1717)

If a rigid body is in equilibrium under a set of P forces and it is subjected to any virtual displacement, the virtual work done by the P forces is zero.

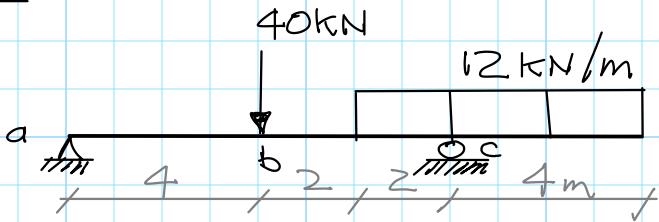
(West, 1998)

$$\sum S^* F = 0$$

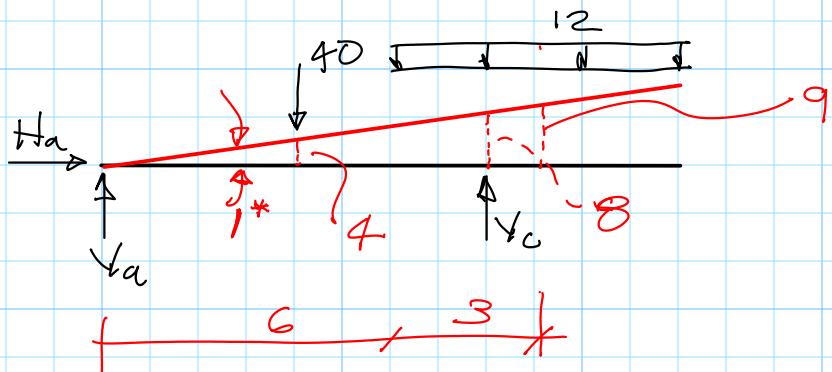


[This agrees with virtual work principle where external work = internal strain energy. Strain energy = 0 for a rigid structure because strain = 0]

Example 1



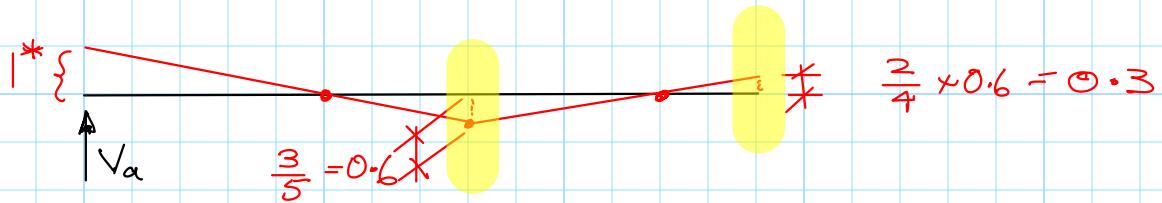
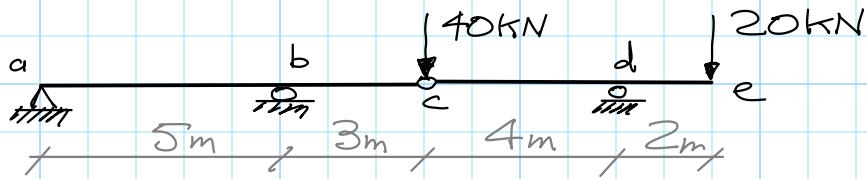
Compute vertical reaction at pt. C.



$$-40 \times 4 + V_c \times 8 - 12 \times 6 \times 9 = 0$$

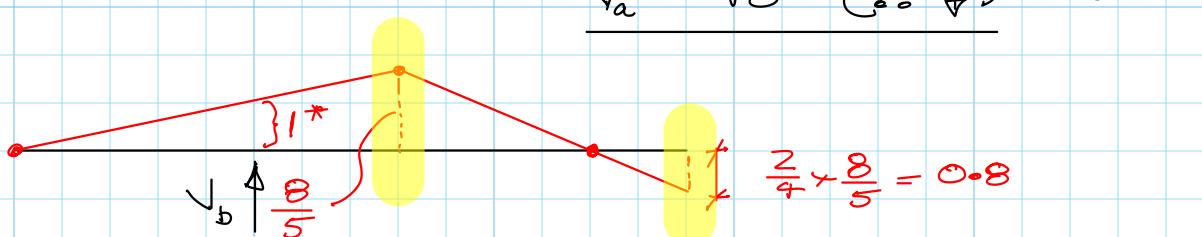
$$V_c = 101 \text{ kN}$$

Example 2 - Compute reactions @ a, b, d



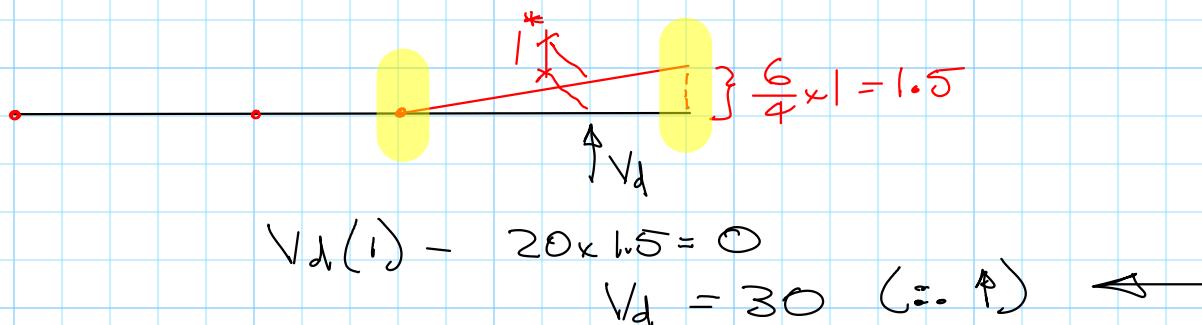
$$V_a(1) + 40 \times 0.6 - 20 \times 0.3 = 0$$

$$\underline{V_a = -18 \text{ (so } \downarrow)}$$



$$V_b(1) - 40(1.6) + 20 \times 0.8 = 0$$

$$\underline{V_b = 48 \text{ (so } \uparrow)}$$

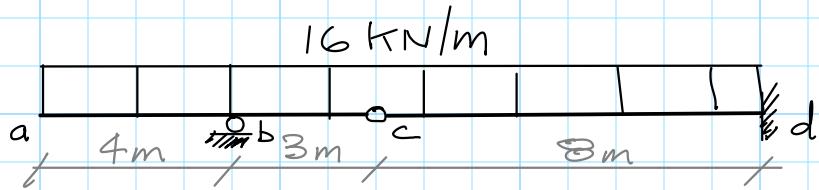


$$V_d(1) - 20 \times 1.5 = 0$$

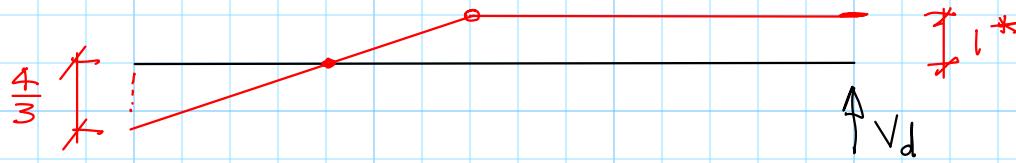
$$\underline{V_d = 30 \text{ (so } \uparrow)}$$

Example 3

- vert reaction & moment @ d

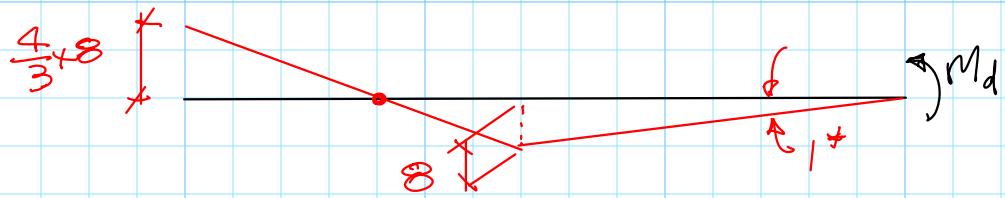


(work done by uniform dist load
= total load x distance moved by centroid)



$$V_d(l) - 16 \times 8 \times l - 16 \times 3 \times l \times \frac{1}{2} + 16 \times 4 \times \frac{4}{3} \times \frac{1}{2} = 0$$

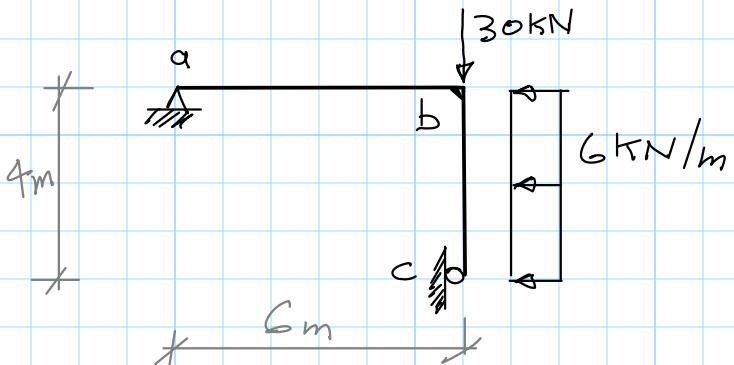
$$\underline{V_d = 109.3 \text{ kN} \quad (\uparrow)}$$



$$M_d(l) + 16 \times 8 \times 8 \times \frac{1}{2} + 16 \times 3 \times 8 \times \frac{1}{2} - 16 \times 4 \times \frac{4}{3} \times 8 \times \frac{1}{2} = 0$$

$$\underline{M_d = -362.7 \quad (\leftarrow)}$$

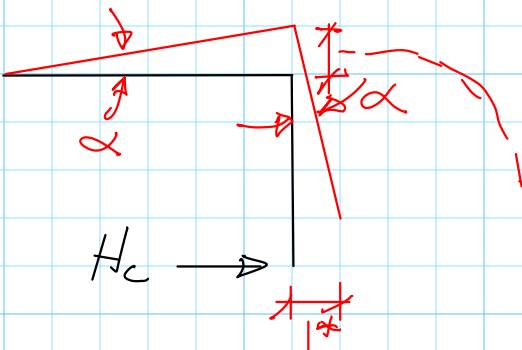
Example 4



Find horizontal reaction
@ pt. C

revised
Nov 4.8/100}

Virtual horizontal displacement of pt. C.



$$4\alpha = 1 \quad \left. \begin{array}{l} \\ \end{array} \right\} \text{from bc}$$

$$\alpha = 1/4 \quad \left. \begin{array}{l} \\ \end{array} \right\} \text{from ab}$$

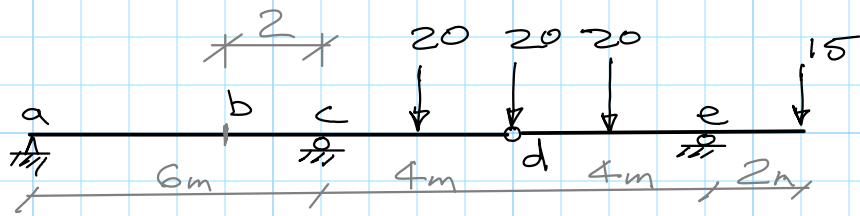
$$6\alpha = \frac{6}{4} = 1.5 \quad \text{from ab}$$

(all portions rotate
equally)

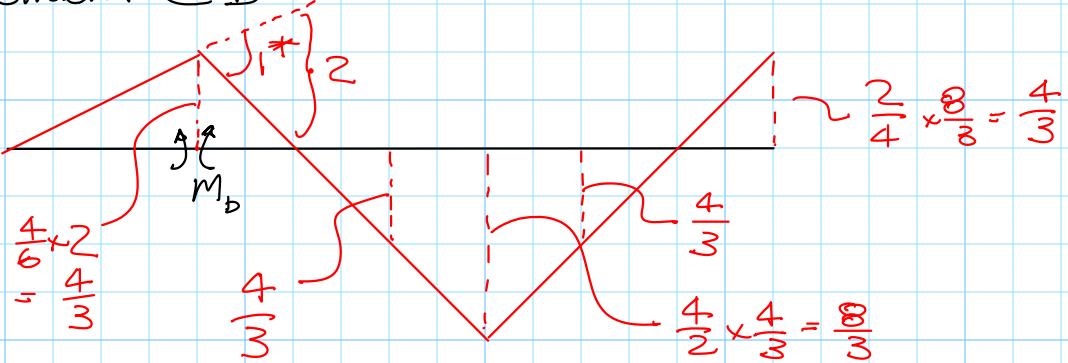
$$H_C(1) - 6 \times 4 \times 1 \times \frac{1}{2} - 30 \times 1.5 = 0$$

$$\underline{H_C = 57 \quad (\rightarrow)}$$

Example 5 - Internal Forces



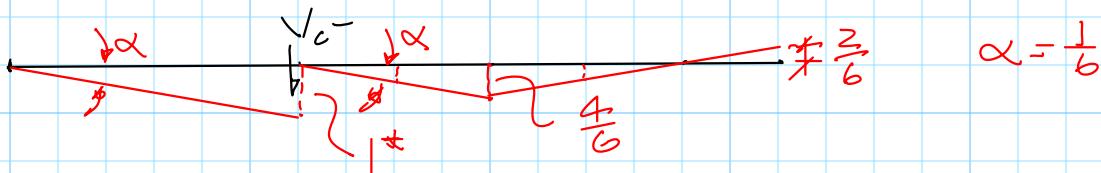
a) moment @ b



$$M_b + 20 \times \frac{4}{3} + 20 \times \frac{8}{3} + 20 \times \frac{4}{3} - 15 \times \frac{4}{3} = 0$$

$$\underline{M_b = -86.7 \quad (\therefore -\downarrow)}$$

b) shear left of c



$$V_c + 20 \times \frac{2}{6} + 20 \times \frac{4}{6} + 20 \times \frac{2}{6} - 15 \times \frac{2}{6} = 0$$

$$\underline{V_c = -21.7 \quad (\therefore -\downarrow)}$$

c) shear right of pin @ d



$$V_d^+ + 20 \times \frac{1}{2} - 15 \times \frac{1}{2}$$

$$\underline{V_d^+ = -2.5 \quad (\therefore -\downarrow)}$$